

TRAFFIC IMPACT STUDY VICDOM BROCK ROAD PIT EXPANSION TOWNSHIP OF UXBRIDGE

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## TOWNSHIP OF UXBRIDGE

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## TRAFFIC IMPACT STUDY

# VICDOM BROCK ROAD PIT EXPANSION TOWNSHIP OF UXBRIDGE 

### 1.0 INTRODUCTION

Vicdom Sand and Gravel (Ontario) Ltd. (Vicdom) has owned an operated a gravel pit just south of Coppins Corners for many years. The existing 220 hectare pit includes three properties licenced under the Aggregate Resources Act, known as the Main Pit, Reagan Pit and Milne Pit. It is located between Brock Road (Regional Road 1) and Concession 4 just south of Regional Road 21as shown on Figure 1 - Location. The existing pits functions as one operation, sharing aggregate processing facilities and one main entrance/exit from Brock Road.

Vicdom's objective is to licence an additional 49 hectares of land in Part of Lots 10 and 11, Concession 4 of the Township of Uxbridge south and east of the existing licences. Following approval of the additional lands, Vicdom intends to amalgamate the new licence with the Main and Regan licences. The Milne pit is nearing depletion and therefore will not be included in the amalgamation.

The existing entrance from Brock Road will serve the amalgamated operation, and no increase in the maximum annual tonnage of material to be extracted from the expanded pit is proposed.

This study is intended to address policies and support applications for amendments to the Region of Durham Official Plan and the Township of Uxbridge Official Plan and Zoning By-law, and the application under the Aggregate Resources Act for licencing of the additional lands.


### 2.0 CONTEXT

The market area for the Brock Road Pit is Durham Region and the eastern GTA particularly southern York Region. Primary haul routes from the pit are shown on Figure 2- Haul Routes. Traffic from the pit travels south along Brock Road other Regional Roads or Provincial Highways or north to Regional Road 21 then west to Highway 47 or east to Regional Road 23 or Highway 7/12. All of these roads are designated Type "A" Arterial Roads, part of the "Strategic Goods Movement Network" in the Durham Region Official Plan, 2008.

The entrance to the pit, located approximately 1 kilometre south of Regional Road 21 was constructed in 1962 In 1984 the entrance was widened and a right turn taper and right turn acceleration lane added. Brock Road in the vicinity of the entrance has a two lane cross section with wide gravel shoulders. The expanded pit will continue to utilize the existing entrance and haul routes.

### 3.0 TRAVEL DEMAND

### 3.1 Horizon Year and Time Period of Analysis

It is assumed for the purposes of this study that the expansion to the pit will be approved in 2012. A study horizon of 10 years to 2019 has been used.

### 3.2 Historic Traffic Volumes

The Region of Durham provides Average Annual Daily Traffic (AADT) volumes for Regional Roads on its web site. The AADT's for Brock Road in two locations are shown on Table 1 and included in Appendix A. The traffic count locations were 200 metres north of Regional Road 5, approximately 7 kilometres south of the site entrance, and between Regional Road 21 and Highway 47 north of the site.

The annual rate of change in the AADT's for Brock Road between 1999/2000 and 2010 varied widely. However, the average change the over the 10 or 11 year period varied from about $6 \%$ south of the site in the City of Pickering to about $4 \%$ north of the site.

Population, and therefore traffic volume, has grown faster in the urban area of Pickering than in the rural area in the vicinity of the site. The "Growth Plan for the Greater Golden Horseshoe, 2006", prepared under the Places to Grow Act 2005, directs that future population growth in Durham be concentrated in the urban centres along Lake Ontario. Therefore, the increase in

STRATEGIC GOODS MOVEMENT NETWORK

traffic volume on Brock Road in the rural area will continue to be less than in the Urban area. It is assumed that the AADT for Brock Road in the vicinity of the site will increase by an average of approximately $3 \%$ through the study period.

Table 1 - Historic Traffic Volumes

| Year | Brock Road |  |
| :--- | :---: | :---: |
|  | 200 m North of <br> RR 5 | 200m South of <br> Hwy 47 Junction |
| 1999 | 4270 |  |
| 2000 |  | 2860 |
| 2001 | 6930 | 3090 |
| 2002 | 7280 | 3740 |
| 2003 | 8330 | 3980 |
| 2004 | 7880 | 4020 |
| 2005 | 8353 | 3591 |
| 2006 | 8190 | 3410 |
| 2007 | 8450 | 3720 |
| 2008 | $6 \%$ | 3920 |
| 2009 | $4 \%$ |  |
| 2010 |  |  |
| Average <br> Annual <br> Increase | 200 |  |

### 3.3 Detailed Traffic Counts

Detailed traffic counts were requested from the Region of Durham for the intersection of Brock Road and Regional Road 21. The counts were taken on Thursday May 14, 2009. This data is included in Appendix A.

The detailed count shows that on Brock Road south of Regional Road 21 (Highway 47) the peak AM hour traffic volume occurred between $7: 15$ and $8: 15$ am when there was a total of 621 vehicles, or $16.7 \%$ of the 2009 AADT. South bound traffic represents $75 \%$ of the total volume during that hour. The PM Peak Hour occurred between 5:00 and 6:00 pm when the total number of vehicles was 654 , or $17.6 \%$ of the 2009 AADT. North bound traffic was $68 \%$ of the total. The
directional distribution of traffic in the AM and PM peak hours suggest that Regional Road 21 is primarily used by commuters from areas to the north, east and west travelling to and from the urban areas to the south.

Given the peak hours for shipping from the pit, as described below, the AM Peak Hour will be analysed as the most relevant condition for the pit expansion. The directional split for the AM Peak Hour is $75 \%$ south bound and $25 \%$ north bound.

### 3.4 Projected Background Traffic

Based on an average growth in traffic volume of $3 \%$, the future traffic volumes on Brock Road are shown in Table 2.

Table 2 - Projected Background Traffic Volumes

| Location | Year | AADT <br> Annual <br> Increase <br> 3\% | Peak Hour |  | Northbound$25 \%$ | Southbound$75 \%$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{gathered} \% \text { of } \\ \text { AADT } \end{gathered}$ | Volume |  |  |
| Pit Entrance | 2010 | 3,920 | 16.7\% | 655 | 164 | 491 |
|  | 2012 | 4,160 | 16.7\% | 695 | 174 | 521 |
|  | 2022 | 5,590 | 16.7\% | 934 | 234 | 700 |

### 3.5 Site Generated Traffic

### 3.5.1 Traffic Volume

Traffic volumes for the operation were calculated based on the operational and transportation information provided by the Bruno Giordano of Vicdom Sand and Gravel.

The amount of material shipped from the site will vary from year to year depending on market conditions. However, it cannot exceed the maximum tonnage permitted by the Licence under the Aggregate Resources Act. The combined licences for the existing operation permit a maximum of $2,165.000$ tonnes per year. No increase in maximum tonnage is requested relative to the expansion. The annual average production is $1,000,000$ Tonnes. Calculations for both the average and maximum tonnage are included in Appendix B.

Eighty percent of the annual tonnage is shipped from April $1^{\text {st }}$ to December $31^{\text {st }}$ each year due to reduced demand in the winter. The traffic generated by the operation is, and will continue, to be comprised primarily of trucks transporting the aggregate products from the pit to customers, with relatively small numbers of employee and service vehicles. Additional truck traffic will be generated by the importation of limestone products for resale.

The truck traffic generated by gravel pits tends to be distributed relatively evenly throughout the day. While the hours of operation for the pit are between 6:00 and 6:00 pm, for the purposes of this analysis it is assumed that $90 \%$ of the daily volume will shipped during the 10 hour period between 6:00 am and 4:00 pm.

The peak traffic volume from the pit is calculated to be 380 vehicles per day or 34 vehicles per hour at the average annual production of $1,000,000$ tonnes and 759 vehicles per day or 71 vehicles per hour at the maximum annual production of $2,165.000$ tonnes. There will be no increase in traffic relative to the expansion of the pit.

### 3.5.2 Directional Distribution

Based on current markets, the current and predicted directional distribution of traffic is $60 \%$ to and from the south and $40 \%$ to and from the north. The total peak hour traffic volumes for average and maximum production with directional distribution are shown in Tables 3A and 3B.

Table 3A - Traffic Distribution at Pit Entrance/Brock Road - Average Production

| Existing Pit (2010) | 34 | 10 | 7 | 7 | 10 |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Total 2010 | 655 | 10 | 7 | 7 | 10 |
| Background Traffic (2012) | 661 |  |  |  |  |
| Expanded Pit (2012) | 34 | 10 | 7 | 7 | 10 |
| Total 2012 | 695 | 10 | 7 | 7 | 10 |
| Background Traffic (2022) | 900 |  |  |  |  |
| Expanded Pit (2022) | 34 | 10 | 7 | 7 | 10 |
| Total 2022 | 934 | 10 | 7 | 7 | 10 |

Table 3B - Traffic Distribution at Pit Entrance/Brock Road - Maximum Production

| Background Traffic (2010) | 621 | Right (60\%) | Left <br> $(40 \%)$ | Right (40\%) | Left <br> $(60 \%)$ |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Existing Pit (2010) | 71 | 21 | 14 | 14 | 21 |
| Total 2010 | 692 | 21 | 14 | 14 | 21 |
| Background Traffic (2012) | 661 |  |  |  |  |
| Expanded Pit (2012) | 71 | 21 | 14 | 14 | 21 |
| Total 2011 | 732 | 21 | 14 | 14 | 21 |
| Background Traffic (2022) | 900 | 71 | 21 | 14 | 14 |
| Expanded Pit (2022) | 971 | 21 | 14 | 14 | 21 |
| Total 2022 |  |  |  |  |  |

### 4.0 EVALUATION OF IMPACTS

### 4.1 Methodology

The intersections of the pit entrance with Brock Road was evaluated using the method described in the Highway Capacity Manual ${ }^{1}$. The level of service definitions area included in Appendix C. "McTrans Traffic Software" was used to carry out the calculations.

The objective of the analysis is to identify "problem" intersections and traffic movements. For rural areas, "problem" intersections and movements are typically defined as those where:

- the overall intersection volume/capacity (v/c) ratio exceeds 0.70 ; or,
- the individual movement v/c ratio exceeds 0.70 ; or,
- an exclusive turning movement generates queues which exceed the available storage space.

Generally, traffic impacts should be mitigated when site generated traffic creates or worsens a "problem" situation.

### 4.2 Analysis

As shown in Tables 3A and 3B, all turning movements related to the Vicdom Brock Road Pit entrance will continue to have a good to fair level of service throughout the study period at the average and maximum production rates. The Summary sheets are included in Appendix D.

The level of service of the intersection based on average production in 2010 is "A" for northbound Brock Road and "B" and "C" for right and left turns respectively from the entrance.

Whereas the level of service of the intersection based on maximum production in 2022 is "A" for northbound Brock Road and "C" for right and left turns from the entrance.

[^0]
### 5.0 CONCLUSIONS

Based on our research and analysis, we conclude the following.

- The existing Vicdom Brock Road Pit generates about 380 vehicles per peak day or 34 vehicles per hour in an average production year, and would generate about 759 vehicles per peak day or 71 vehicles per hour at the maximum annual tonnage permitted by the pit licences.
- No increase in production will occur as a result of the expansion of the pit.
- Traffic generated by the operation is and will continue to be primarily trucks transporting aggregate materials to the current markets in the southern Durham and eastern GTA areas.
- Traffic to and from the pit uses, and will continue to primarily use, Regional Roads and Provincial Highways that are part of the Strategic Goods Movement Network in the Region.
- All of the turning movements at the intersection of the Vicdom Pit entrance and Brock Road will continue to operate at a good to fair level of service throughout the study period.

Therefore, no mitigation measures are warranted or proposed.

All of which is respectfully submitted,
SKELTON, BRUMWELL \& ASSOCIATES INC.
per:


Scott W. Brumwell, P.Eng.
Vice President

## APPENDIX A

Traffic Count Information

REGIONAL RD 1 @ REGIONAL RD 21
Uxbridge



## APPENDIX B

Site Generated Traffic

# APPENDIX B <br> SITE GENERATED TRAFFIC <br> VICDOM BROCK ROAD PIT 

## PIT AGGREGATE

## Production

| Average | $1,000,000$ | tonnes |
| :--- | ---: | ---: |
| Maximum | $2,165,000$ | tonnes |


| Fleet Usage | Tonnes Per Load | \% of Trips |
| :--- | ---: | ---: |
| Triaxle | 23 | $40 \%$ |
| Tractor with Trailer/ |  |  |
| Triaxle with Pony | 35 | $60 \%$ |

Average per Load 30.2 tonnes
Annual Trip Generation

|  | Total Annual <br> Tonnage | Tonnes Per Load | Trips Per Year |
| :--- | :---: | :---: | :---: |
| Average Year | $1,000,000$ |  | 30.2 |
|  |  | Total Trips Out | 33,113 |
|  |  | Total Trips In | 33,113 |
|  |  | Total Trips (Out + In) | 66,225 |
| Maximum Year | $2,165,000$ |  |  |
| (Licence Limit) |  | 30.2 | 71,689 |
|  |  | Total Trips Out | 71,689 |
|  |  | Total Trips In | 71,689 |

## Daily Trip Generation

Percentage (\%) shipped in peak months 80\%
Peak Months: April to December
9
Average number of working days per month: 21

| Total Annual Tonnage | Total Trips Per <br> Year | Total Working Days <br> in Peak Months | Trips Per Day |
| :---: | :---: | :---: | :---: |
| $1,000,000$ | 66,225 | 189 |  |
| $2,165,000$ | 143,377 | 189 | 350 |

## APPENDIX B <br> SITE GENERATED TRAFFIC <br> VICDOM BROCK ROAD PIT

## IMPORTED LIMESTONE

| Annual Tonnage | 100,000 | tonnes |
| :--- | :---: | :--- |
| Tonnage per Load | 35 | tonnes |

Annual Trip Generation

|  | Total Annual | Tonnes Per Load | Trips Per Year |
| :---: | :---: | :---: | :---: |
| Tonnage |  |  |  |
| 100,000 | 35 | 2,857 |  |
|  |  | Total Trips Out | 2,857 |
|  |  | Total Trips In | 2,857 |
|  |  | Total Trips (Out $+\ln )$ | 5,714 |

Daily Trip Generation

| Total Annual Tonnage | Total Trips Per <br> Year | Total Working Days <br> in Peak Months | Trips Per Day |
| :---: | :---: | :---: | :---: |
| 100,000 | 5,714 | 189 | 30 |

## TOTAL TRIP GENERATION

Peak Hours of Operation
From: 6:00 AM

To: 4:00 PM
Total Hours: 10
\% Shipped in Peak Hours
90\%

| Total Annual Tonnage | Trips Per Day | Average Trips Per <br> Peak Hour | Minutes Between <br> Trips |
| :---: | :---: | :---: | :---: |
| $1,000,000$ | 380 | 34 | 2 |
| $2,165,000$ | 789 | 71 | 1 |

## APPENDIX C

## Level of Service Definition

## LEVEL OF SERVICE AT UNSIGNALIZED INTERSECTIONS

The assessment of unsignalized intersections is based on the method described in the "Highway Capacity Manual, Special Report 209", published in 1994 by the Transportation Research Board.

The term "Level of Service" is often used to assist in clarifying the arithmetic analysis associated with traffic engineering. "Level of Service" implies a qualitative measure of traffic flow at an intersection, and is dependent upon vehicle delay and vehicle queue lengths at the approaches. The Level of Service can be determined based on the ratio between traffic volumes and approach capacity or "V/C" ratio. The following table describes the characteristics of each level:

| Level of Service | Description | Avg. Stop Delay(s) | V/C <br> Ratio |
| :---: | :---: | :---: | :---: |
| A | Little or no traffic delay occurs. Approaches appear open, turning movements are easily made, and drivers have freedom of operation. | $\leq 5.0$ | 0-0.59 |
| B | Short traffic delays occur. Many drivers begin to feel somewhat restricted in terms of freedom of operation. | 5.0-15.0 | $\begin{gathered} 0.60- \\ 0.69 \end{gathered}$ |
| C | Average traffic delays occur. Operations are generally stable, but drivers emerging from the minor street may experience difficulty in completing their movement. This may occasionally impact on the stability of flow on the major street. | $\begin{gathered} 15.0- \\ 25.0 \end{gathered}$ | $\begin{aligned} & 0.70- \\ & 0.79 \end{aligned}$ |
| D | Long traffic delays occur. Motorists emerging from the minor street experience significant restriction and frustration. Drivers on the major street will experience congestion and delay as drivers emerging from the minor street interfere with the major through movements. | $\begin{gathered} 25.0- \\ 40.0 \end{gathered}$ | $\begin{gathered} 0.80- \\ 0.89 \end{gathered}$ |
| E | Very long traffic delays occur. Operations approach the capacity of the intersection. | $\begin{gathered} 40.0- \\ 60.0 \end{gathered}$ | $\begin{gathered} 0.90- \\ 0.99 \end{gathered}$ |
| F | Saturation occurs, with vehicle demand exceeding the available capacity. Extremely long traffic delays occur. | 260.0 | $\geq 1.00$ |

## APPENDIX D

Intersection Analysis


Vehicle Volumes and Adjustments

| Major Street | Northbound |  |  | Southbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | 1 | 2 | 3 | 4 | 5 | 6 |
|  | L | T | R | L | T | R |
| Volume (veh/h) | 10 | 155 |  |  | 466 | 7 |
| Peak-Hour Factor, PHF | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Hourly Flow Rate, HFR (veh/h) | 10 | 155 | 0 | 0 | 466 | 7 |
| Percent Heavy Vehicles | 10 | -- | -- | 0 | - | -- |
| Median Type | Undivided |  |  |  |  |  |
| RT Channelized |  |  | 0 |  |  | 1 |
| Lanes | 0 | 1 | 0 | 0 | 1 | 1 |
| Configuration | LT |  |  |  | $T$ | $R$ |
| Upstream Signal |  | 0 |  |  | 0 |  |
| Minor Street | Eastbound |  |  | Westbound |  |  |
| Movement | 7 | 8 | 9 | 10 | 11 | 12 |
|  | L | T | R | L | T | R |
| Volume (veh/h) | 7 |  | 10 |  |  |  |
| Peak-Hour Factor, PHF | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Hourly Flow Rate, HFR (veh/h) | 7 | 0 | 10 | 0 | 0 | 0 |
| Percent Heavy Vehicles | 100 | 0 | 100 | 0 | 0 | 0 |
| Percent Grade (\%) | -2 |  |  | 0 |  |  |
| Flared Approach |  | N |  |  | $N$ |  |
| Storage |  | 0 |  |  | 0 |  |
| RT Channelized |  |  | 1 |  |  | 0 |
| Lanes | 1 | 0 | 1 | 0 | 0 | 0 |
| Configuration | $L$ |  | $R$ |  |  |  |

Delay, Queue Length, and Level of Service

| Approach | Northbound | Southbound | Westbound |  |  | Eastbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | 1 | 4 | 7 | 8 | 9 | 10 | 11 | 12 |
| Lane Configuration | LT |  |  |  |  | L |  | $R$ |
| $v$ (veh/h) | 10 |  |  |  |  | 7 |  | 10 |
| C (m) (veh/h) | 1055 |  |  |  |  | 336 |  | 449 |
| V/c | 0.01 |  |  |  |  | 0.02 |  | 0.02 |
| 95\% queue length | 0.03 |  |  |  |  | 0.06 |  | 0.07 |
| Control Delay (s/veh) | 8.4 |  |  |  |  | 15.9 |  | 13.2 |
| LOS | A |  |  |  |  | C |  | B |
| Approach Delay (s/veh) | - | -- |  |  |  | 14.3 |  |  |
| Approach LOS | -- | -- |  |  |  |  | B |  |



APPENDIX E
Curriculum Vitae
Scott Brumwell, P. Eng.

# Scott W. Brumwell, B.Sc. (Eng.), P. Eng. Vice President, Principal 

## EDUCATION

## Bachelor of Science in Engineering

University of Guelph, 1983
Majored in Water Resources Engineering

## PROFESSIONAL BACKGROUND

## Skelton, Brumwell \& Associates Inc.

1987 to present

Vice President and Principal Engineer responsible for coordination of various municipal engineering projects undertaken by the firm. Specializing in development servicing design (roads, sewers, watermains), master servicing planning, stormwater management, transportation impact analysis and Phase I Environmental Site Assessments.

## R. E. Clipsham Limited

1983 to 1987

Project Engineer responsible for the preparation of designs, reports, cost estimates and tender documents for various municipal engineering projects undertaken by the firm.

## MEMBERSHIP \& ASSOCIATIONS

Professional Engineers of Ontario (designated as a Consulting Engineer)
Qualified Designer for Sewage Systems and Plumbing (All Buildings) under Section 2.17 of the Ontario Building Code (BCIN 24241)

Institute of Transportation Engineers
Canadian Water Resources Association

Kempenfelt Rotary Club
Chairman of the Simcoe County Chapter Executive of the Professional Engineers of Ontario (1990-1991)

# Scott W. Brumwell, B.Sc. (Eng.), P. Eng. Vice President, Principal 

## WORK RELATED COURSES

Consulting Engineers of Ontario
and Ontario Ministry of Natural Resources
Urban Drainage Design, 1988

Ministry of the Environment
Implementation of pollution control measures for urban stormwater runoff, 1989

Ministry of Transportation
New MTO Drainage Management Policy and Practice, 1989

The Canadian Institute
Subdividing Land, 1990

The Canadian Institute
Effluent Management for the 1990's, 1990

Technical University of Nova Scotia
Stormwater Management, 1991
Executive Enterprises Inc.

Effective Strategies for Environmental Site Assessments and Cleanup, 1993.

Ministry of the Environment
Stormwater Management Practices and Planning, 1994

University of Toronto
Preparation and Review of Traffic Impact Studies, 1994

University of Toronto
Environmental Legislation and Auditing, 1996

Ministry of the Environment
Stormwater / CSO Technology Transfer Conference, 1998

Ontario Traffic Conference
Rural Roadway Safety Initiatives, 2005


[^0]:    ${ }^{1}$ Highway Capacity Manual Special Report 209 Third Edition@ Transportation Research Board, National Research Council, Washington, D.C., 1998.

